## Example 7.2 Rectangular foundation subjected to eccentric loading

## 1 Description of the problem

For comparison with complex foundation shape, no analytical solution is yet available. Therefore, for judgment on the nonlinear analysis of foundations for simple assumption model, consider the rectangular foundation shown in Figure 7.5. The foundation has the length $L=8.0[\mathrm{~m}]$ and the width $B=6.0[\mathrm{~m}]$. The foundation carries an eccentric load of $N=2000[\mathrm{kN}]$. Both of the $x$-axis and $y$-axis are main axes, which intersect in the center of gravity $s$ of the foundation area. The position of resultant $N$ is defined by the ordinates $x=e_{x}$ and $y=e_{y}$. Within the rectangle foundation area five zones are represented. It is found that the contact area and maximum corner pressure $\max q_{o}$ depends on the position of the resultant $N$ in these five zones (Irles/ Irles (1994)). In this example, the maximum corner pressure max $q_{o}$ is obtained using the program ELPLA for each zone and compared with other analytical salutations, which are available for rectangular foundation.


Figure 7.5 Division of the rectangular foundation in five zones according to the position of the resultant $N$

## 2 Hand calculation of the maximum corner pressure max $q_{o}$

The maximum corner pressure max $q_{o}$ for the zone (1) can be obtained directly using Equation 7.1, where in this case the Resultant $N$ lies in the foundation kern and no separation will occur. The maximum corner pressure $\max q_{o}$ for the other four zones can be obtained using available analytical solutions according to Irles/ Irles (1994), Teng (1962) and Graßhoff/ Kany (1997) as follows:

Zone (2)
Three corners detached ( $\left.e_{x}=3.0[\mathrm{~m}], e_{y}=2.25[\mathrm{~m}]\right)$
The maximum corner pressure max $q_{o}$ for zone (2), Figure 7.6a, can be given according to Irles/ Irles (1994) from the following equation

$$
\begin{gathered}
\max q_{o}=\frac{3 N}{2\left(L-2 e_{x}\right)\left(B-2 e_{y}\right)} \\
\max q_{o}=\frac{3 \times 2000}{2(8-2 \times 3)(6-2 \times 2.25)}=1000\left[\mathrm{kN} / \mathrm{m}^{2}\right]
\end{gathered}
$$

## Zone (3)

Two corners detached ( $\left.e_{x}=3.0[\mathrm{~m}], e_{y}=0.0[\mathrm{~m}]\right)$
The maximum corner pressure max $q_{o}$ for zone (3), Figure 7.6b, can be given according to Teng (1962) from the following equation

$$
\begin{gathered}
\max q_{o}=\frac{N}{L B}\left(\frac{4 L}{3 L-6 e_{x}}\right) \\
\max q_{o}=\frac{2000}{8 \times 6}\left(\frac{4 \times 8}{3 \times 8-6 \times 3}\right)=222.22\left[\mathrm{kN} / \mathrm{m}^{2}\right]
\end{gathered}
$$

## Zone (4)

Two corners detached ( $\left.e_{x}=1.0[\mathrm{~m}], e_{y}=2.25[\mathrm{~m}]\right)$
The maximum corner pressure max $q_{o}$ for zone (4), Figure 7.6c, can be given according to Graßhoffl Kany (1997) from the following equation

$$
\begin{gathered}
t=\frac{L}{12}\left(\frac{L}{e_{x}}+\sqrt{\frac{L^{2}}{e_{x}{ }^{2}}-12}\right)=\frac{8}{12}\left(\frac{8}{1.0}+\sqrt{\frac{8^{2}}{1.0^{2}}-12}\right)=10.141[\mathrm{~m}] \\
\tan \beta=\frac{3}{2}\left(\frac{B-2 e_{x}}{t+e_{x}}\right)=\frac{3}{2}\left(\frac{6-2 \times 2.25}{10.141+1.0}\right)=0.202 \\
\max q_{o}=\frac{12 N}{L \tan \beta} \frac{L+2 t}{L^{2}+12 t^{2}}=\frac{12 \times 2000}{8 \times 0.202} \frac{8+2 \times 10.141}{8^{2}+12 \times 10.141^{2}}=323.58\left[\mathrm{kN} / \mathrm{m}^{2}\right]
\end{gathered}
$$

## Zone (5)

Only one corner detached ( $e_{x}=1.0[\mathrm{~m}], e_{y}=0.75[\mathrm{~m}]$ )
The maximum corner pressure max $q_{o}$ for zone (5), Figure 7.6d, can be given according to Graßhoffl Kany (1997) from the following equation

$$
\begin{gathered}
K=\frac{e_{x}}{L}+\frac{e_{y}}{B}=\frac{1.0}{8}+\frac{0.75}{6}=0.25 \\
\max q_{o}=\frac{N}{L B} K[12-3.9(6 K-1)(1-2 K)(2.3-2 K)] \\
\max q_{o}=\frac{2000}{8 \times 6} K[12-3.9(6 \times 0.25-1)(1-2 \times 0.25)(2.3-2 \times 0.25)] \\
\max q_{o}=106.72\left[\mathrm{kN} / \mathrm{m}^{2}\right]
\end{gathered}
$$



Figure 7.6 Resultant $N$ lies in the four zones (2) to (5)

## 3 Determination of the maximum corner pressure max $q_{o}$ by the program ELPLA

To achieve the comparison between the maximum corner pressure max $q_{o}$ obtained from the program ELPLA and that obtained from the other available analytical solutions described above, the rectangular foundation is subdivided into refine mesh of square finite elements. Each element has a side of $0.1[\mathrm{~m}]$. The results obtained from the program ELPLA are compared with those obtained above in Table 7.3. It shows that the results of both the analytical and iteration methods are in a good agreement.

Table 7.3 Comparison between the maximum corner pressure max $q_{o}\left[\mathrm{kN} / \mathrm{m}^{2}\right]$ obtained from program ELPLA and that obtained from the available analytical solutions

| Zone No. | Zone (2) | Zone (3) | Zone (4) | Zone (5) |
| :---: | :---: | :---: | :---: | :---: |
| Available <br> solutions | Irles/ Irles (1994) | Teng (1962) | Graßhoff/ Kany (1997) |  |
|  | 1000 | 222 | 324 | 107 |
| ELPLA | 1017 | 223 | 325 | 106 |
| Difference [\%] | 1.67 | 0.45 | 0.31 | 0.94 |

